

# FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE \_\_\_\_\_, FIELD EVALUATION DATE \_\_\_\_\_  
PROPERTY OWNER: \_\_\_\_\_ PHONE \_\_\_\_\_  
ADDRESS: \_\_\_\_\_ CITY, STATE, ZIP: \_\_\_\_\_  
LEGAL DESCRIPTION: \_\_\_\_\_  
PIN# \_\_\_\_\_ SEC \_\_\_\_\_ T \_\_\_\_\_ R \_\_\_\_\_ TWP NAME \_\_\_\_\_  
FIRE# \_\_\_\_\_ LAKE/RIVER \_\_\_\_\_ LAKE CLASS \_\_\_\_\_ OHWL \_\_\_\_\_ FT.

## DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2	REFERENCE BM ELEV. _____ FT.
DISTURBED AREAS	YES ___ NO ___	YES ___ NO ___	REFERENCE BM DESCRIPTION _____
COMPACTED AREAS	YES ___ NO ___	YES ___ NO ___	_____
FLOODING	YES ___ NO ___	YES ___ NO ___	_____
RUN ON POTENTIAL	YES ___ NO ___	YES ___ NO ___	_____
SLOPE %	_____	_____	_____
DIRECTION OF SLOPE	_____	_____	_____
LANDSCAPE POSITION	_____	_____	_____
VEGETATION TYPES	_____	_____	_____

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 \_\_\_\_\_, 1A \_\_\_\_\_, 2 \_\_\_\_\_, 2A \_\_\_\_\_

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 \_\_\_\_\_ FT., #2 \_\_\_\_\_ FT.

SOIL SIZING FACTOR: SITE # 1 \_\_\_\_\_, SITE #2 \_\_\_\_\_

CONSTRUCTION RELATED ISSUES: \_\_\_\_\_

LIC# \_\_\_\_\_ SITE EVALUATOR SIGNATURE: \_\_\_\_\_

SITE EVALUATOR NAME: \_\_\_\_\_ TELEPHONE# \_\_\_\_\_

LUG REVIEW \_\_\_\_\_ DATE \_\_\_\_\_

Comments: \_\_\_\_\_

SOIL BORING LOGS ON REVERSE SIDE

# SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

1 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

# TRENCH AND BED WORKSHEET

## 1. AVERAGE DESIGN FLOW

- A. Estimated \_\_\_\_\_ gpd (see figure A-1)  
 or measured  $\times 1.5$  (safety factor) = \_\_\_\_\_ gpd
- B. Septic tank capacity \_\_\_\_\_ gal (see figure C-1)

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

## 2. SOILS (Site evaluation data)

- C. Depth to restricting layer = \_\_\_\_\_ ft
- D. Max depth of system Item 2C - 3 ft = \_\_\_\_\_ ft - 3 ft = \_\_\_\_\_ ft
- E. Texture \_\_\_\_\_ Percolation rate \_\_\_\_\_ MPI
- F. Soil Sizing Factor (SSF) \_\_\_\_\_ sqft/gpd (see figure D-15)
- G. % Land Slope \_\_\_\_\_ %

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

## 3. TRENCH or BED BOTTOM AREA

- H. For trenches with 6 inches of rock below the pipe:  
 $A \times F = \text{_____ gpd} \times \text{_____ sqft/gpd} = \text{_____ sqft}$
- I. For trenches with 12 inches of rock below the pipe:  
 $A \times F \times 0.8 = \text{_____ gpd} \times \text{_____ sqft/gpd} \times 0.8 = \text{_____ sqft}$
- J. For trenches with 18 inches of rock below the pipe:  
 $A \times F \times 0.66 = \text{_____ gpd} \times \text{_____ sqft/gpd} \times 0.66 = \text{_____ sqft}$
- K. For trenches with 24 inches of rock below the pipe:  
 $A \times F \times 0.6 = \text{_____ gpd} \times \text{_____ sqft/gpd} \times 0.6 = \text{_____ sqft}$
- L. For gravity beds with 6 or 12 inches of rock below the pipe;  
 $1.5 \times A \times F = 1.5 \times \text{_____ gpd} \times \text{_____ sqft/gpd} = \text{_____ sqft}$   
 For pressure beds with 6 or 12 inches of rock below the pipe;  
 $A \times F = \text{_____ gpd} \times \text{_____ sqft/gpd} = \text{_____ sqft}$

Percolation Rate (minutes per inch (mpi))	Soil Texture	Soil Sizing Factor (square feet/gallon per day (sqft/gpd))
faster than 0.1*	Coarse sand	0.83
0.1 to 5*	Medium sand	0.83
	Loamy sand	
	Fine sand	1.67
0.1 to 5**	Sandy loam	1.27
6 to 15	Loam	1.67
16 to 30	Silt loam	2.00
31 to 45	Silt	
46 to 60	Clay loam	2.20
	Sandy clay	
	Silty clay	
	Clay	4.20
over 61 to 120***	Sandy clay	
	Silty clay	
slower than 120****		

\*Use systems for rapidly permeable soils: pressure distribution or serial distribution with no trench > 25% of the total system.  
 \*\*Soil having 50% or more fine sand plus very fine sand.  
 \*\*\*A mound must be used.  
 \*\*\*\*An other or performance system must be used

## 4. DISTRIBUTION (Check all that apply)

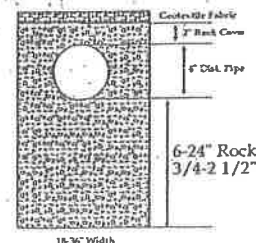
- \_\_\_\_\_ Bed (< 6% slope) \_\_\_\_\_ Drop boxes (any slope) \_\_\_\_\_ Rock
- \_\_\_\_\_ Trenches \_\_\_\_\_ Distribution box (< 3%) \_\_\_\_\_ Chamber
- \_\_\_\_\_ Pressure \_\_\_\_\_ Gravity \_\_\_\_\_ Gravelless

## 5. SYSTEM WIDTH, LENGTH and VOLUME

- M. Select trench width = \_\_\_\_\_ ft
- N. If using rock, divide bottom area by width:  $(H, I, J, K \text{ or } L) \div M = \text{_____ sqft} \div \text{_____ ft} = \text{_____ lineal feet}$   
 Rock depth below distribution pipe plus 0.5 foot times bottom area:  
 Rock depth in feet + 0.5 feet x Area (H, I, J, K, or L)  
 $(\text{_____ ft} + 0.5 \text{ ft}) \times \text{_____ sqft} = \text{_____ cuft}$   
 Volume in cubic yards =  $\text{cuft} \div 27$   
 $\text{_____ cuft} \div 27 = \text{_____ cu yds}$   
 Weight of rock in tons =  $\text{cubic yds} \times 1.4$   
 $\text{_____ cu yds} \times 1.4 = \text{_____ tons}$
- O. If using 10" Gravelless Pipe, Flow (A) x Gravelless SSF (see figure D-9)  
 $\text{_____ gpd} \times \text{_____ lineal feet/gpd} = \text{_____ lineal feet}$
- P. If using Chambers, H, I, J, or K (based on height of chamber slats)  $\div$  width of chamber in feet (M)  
 $\text{_____ sqft} \div \text{_____ ft} = \text{_____ lineal ft}$

percolation rate (minutes/inch)	soil texture	lineal feet/gallon/day
Faster than 0.1*	Coarse Sand	---
0.1 to 5	Medium Sand	0.28
	Loamy Sand	
	Fine Sand**	0.6
0.1 to 5	Sandy Loam	0.42
6 to 15	Loam	0.56
16 to 30	Silt Loam	0.67
31 to 45	Silt	
46 to 60	Clay Loam (CL)	0.74
	Sandy CL	
	Silty CL	
	Clay	---
slower than 60***	Sandy Clay	
	Silty Clay	

\*Soil too coarse for sewage treatment.  
 Use systems for rapidly permeable soils.  
 \*\*Soil having 50% or more fine sand + very fine sand.  
 \*\*\*Soil with too high a percentage of clay for installation of a standard inground system.



## 6. LAWN AREA

- Q. Select trench spacing, center to center = \_\_\_\_\_ feet
- R. Multiply trench spacing by lineal feet  $R \times Q = \text{sqft of lawn area}$   
 $\text{_____ ft} \times \text{_____ ft} = \text{_____ sqft}$

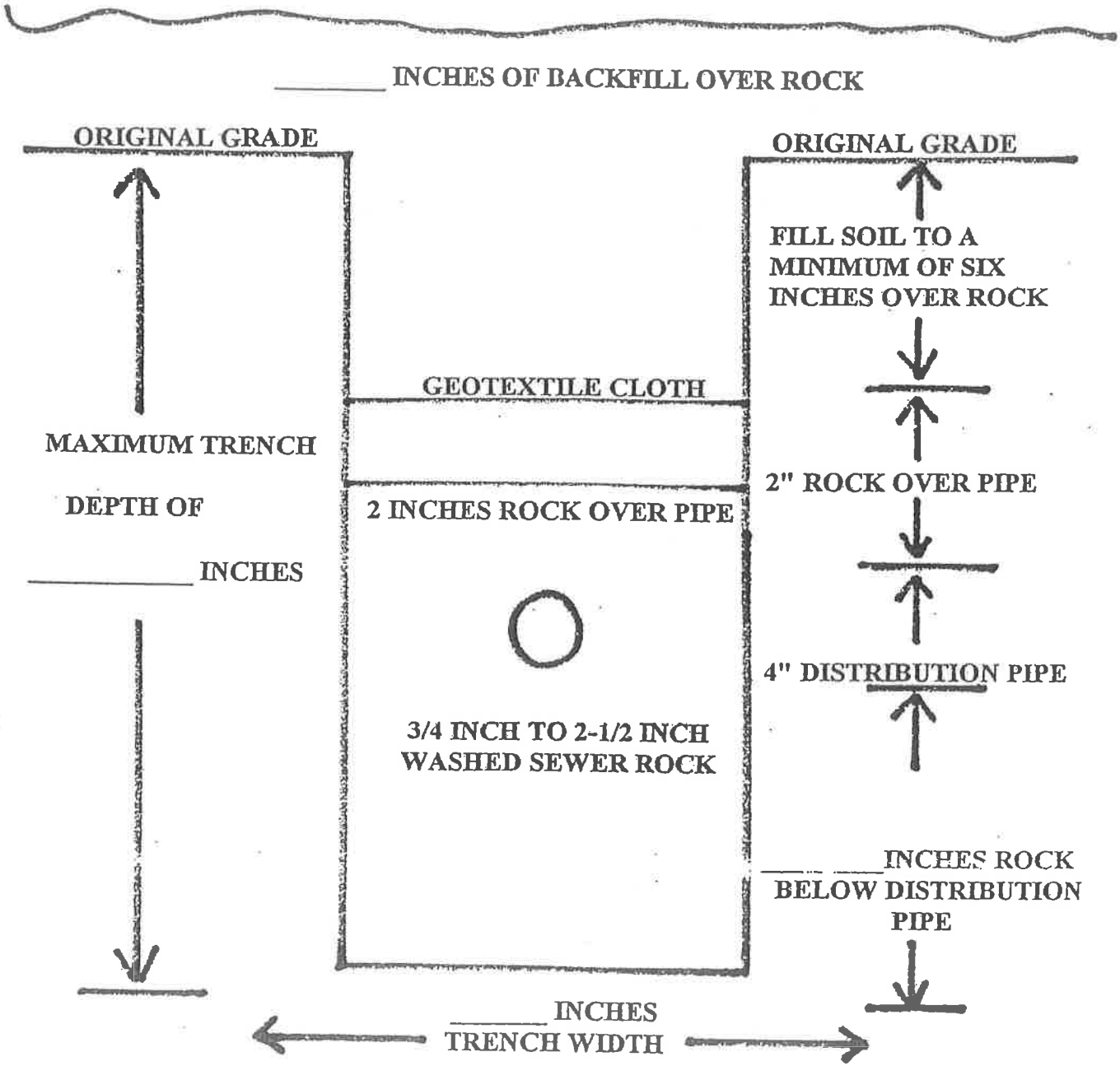
7. Include a drawing with scale (one inch = \_\_\_\_\_ ft). Show pertinent boundaries, right of way, easements, location of house, garage, driveway, all other improvements, existing or proposed soil treatment system, well and dimensions of all elevations, setbacks and separation distances.

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

\_\_\_\_\_ (signature) \_\_\_\_\_ (license #) \_\_\_\_\_ (date)

TRENCH CROSS-SECTION

FINISHED GRADE



# MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

## A. Average Design FLOW

Estimated \_\_\_\_\_ gpd (see figure A-1)  
 or measured \_\_\_\_\_ x 1.5 (safety factor) = \_\_\_\_\_ gpd

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

## B. SEPTIC TANK Capacity

\_\_\_\_\_ gallons (see figure C-1)

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

## C. SOILS (refer to site evaluation)

- Depth to restricting layer = \_\_\_\_\_ feet
- Depth of percolation tests = \_\_\_\_\_ feet
- Texture \_\_\_\_\_  
 Percolation rate \_\_\_\_\_ mpi
- Soil loading rate \_\_\_\_\_ gpd/sqft (see figure D-33)
- Percent land slope \_\_\_\_\_%

## D. ROCK LAYER DIMENSIONS

- Multiply average design flow (A) by 0.83 to obtain required rock layer area.  
 \_\_\_\_\_ gpd x 0.83 sqft/gpd = \_\_\_\_\_ sqft
- Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)  
 0.83 sqft/gpd x \_\_\_\_\_ gpd/sqft = \_\_\_\_\_ ft
- Length of rock layer = area ÷ width =  
 \_\_\_\_\_ sqft (D1) ÷ \_\_\_\_\_ ft (D2) = \_\_\_\_\_ ft

### Mound LLR

< 120 MPI	≤ 12
≥ 120 MPI	≤ 6

## E. ROCK VOLUME

- Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock  
 \_\_\_\_\_ sqft x 1 ft = \_\_\_\_\_ cuft
- Divide cuft by 27 cuft/cuyd to get cubic yards  
 \_\_\_\_\_ cuft ÷ 27 cuft/cuyd = \_\_\_\_\_ cuyd
- Multiply cubic yards by 1.4 to get weight of rock in tons  
 \_\_\_\_\_ cuyd x 1.4 ton/cuyd = \_\_\_\_\_ tons

## F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33)  
 times rock layer width (D2)

\_\_\_\_\_ x \_\_\_\_\_ ft = \_\_\_\_\_ ft

D-33: Absorption Width Sizing Table

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
6 to 15	Sandy Loam	0.79	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam	0.50	2.40
46 to 60	Silt Sandy Clay Loam Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.24	5.00
Slower than 120*			

\*System designed for these soils must be other or performance

**G. MOUND SLOPE WIDTH & LENGTH**  
(landslope greater than 1%)

1. Downslope absorption width = absorption width (F) minus rock layer width (D2)  
\_\_\_\_\_ ft - \_\_\_\_\_ ft = \_\_\_\_\_ ft

2. Calculate mound size  
**UPSLOPE**

a. Depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)  
3 ft - \_\_\_\_\_ ft = \_\_\_\_\_ ft

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)  
\_\_\_\_\_ ft + 1ft + 1ft = \_\_\_\_\_ ft

c. Upslope berm multiplier based on land slope \_\_\_\_\_ (see figure D-34)

d. Upslope width = berm multiplier (G2c) x upslope mound height (G2b):  
\_\_\_\_\_ x \_\_\_\_\_ ft = \_\_\_\_\_ ft

**DOWNSLOPE**

e. Drop in elevation = rock layer width (D2) x percent landslope (C5) ÷ 100  
\_\_\_\_\_ ft x \_\_\_\_\_ % ÷ 100 = \_\_\_\_\_ ft

f. Downslope mound height = depth of clean sand for slope difference (G2e) at downslope rock edge plus the mound height at the upslope edge of rock layer (G2b)  
\_\_\_\_\_ ft + \_\_\_\_\_ ft = \_\_\_\_\_ ft

g. Downslope berm multiplier based on percent land slope \_\_\_\_\_ (see figure D-34)

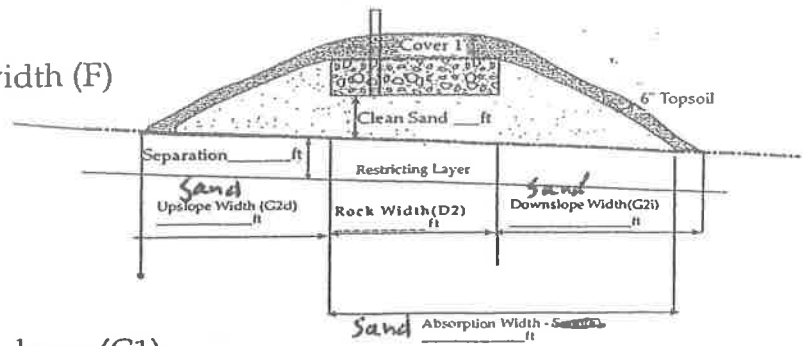
h. Downslope width = downslope multiplier (G2g) times downslope mound height (G2f)  
\_\_\_\_\_ x \_\_\_\_\_ ft = \_\_\_\_\_ ft

i. Select the greater of G1 and G2h as the downslope width: \_\_\_\_\_ ft

j. Total mound width is the sum of upslope width (G2d) width plus rock layer width (D2) plus downslope width (G2i)  
\_\_\_\_\_ ft + \_\_\_\_\_ ft + \_\_\_\_\_ ft = \_\_\_\_\_ ft

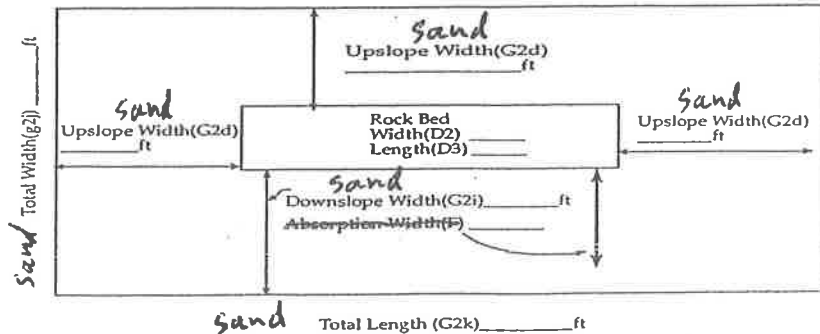
k. Total mound length is the sum of upslope width (G2d) plus rock layer length (D3) plus upslope width (G2d)  
\_\_\_\_\_ ft + \_\_\_\_\_ ft + \_\_\_\_\_ ft = \_\_\_\_\_ feet

Landslope > 1% slope



D-34: SLOPE MULTIPLIER TABLE

Land Slope in %	UPSLOPE multipliers for various slope ratios						DOWNSLOPE multipliers for various slope ratios				
	3:1	4:1	5:1	6:1	7:1	8:1	3:1	4:1	5:1	6:1	7:1
0	3.0	4.0	5.0	6.0	7.0	8.0	3.0	4.0	5.0	6.0	7.0
1	2.91	3.85	4.76	5.66	6.54	7.41	3.09	4.17	5.26	6.38	7.53
2	2.83	3.70	4.54	5.36	6.14	6.90	3.19	4.35	5.56	6.82	8.14
3	2.75	3.57	4.35	5.08	5.79	6.45	3.30	4.54	5.88	7.32	8.86
4	2.68	3.45	4.17	4.84	5.46	6.06	3.41	4.76	6.25	7.89	9.72
5	2.61	3.33	4.00	4.62	5.19	5.71	3.53	5.00	6.67	8.57	10.77
6	2.54	3.23	3.85	4.41	4.93	5.41	3.66	5.26	7.14	9.38	12.07
7	2.48	3.12	3.70	4.23	4.70	5.13	3.80	5.56	7.69	10.34	13.73
8	2.42	3.03	3.57	4.05	4.49	4.88	3.95	5.88	8.33	11.54	15.91
9	2.36	2.94	3.45	3.90	4.30	4.65	4.11	6.25	9.09	13.04	18.92
10	2.31	2.86	3.33	3.75	4.12	4.44	4.29	6.67	10.00	15.00	23.33
11	2.26	2.78	3.23	3.61	3.95	4.26	4.48	7.14	11.11	17.65	30.43
12	2.21	2.70	3.12	3.49	3.80	4.08	4.69	7.69	12.50	21.43	43.75

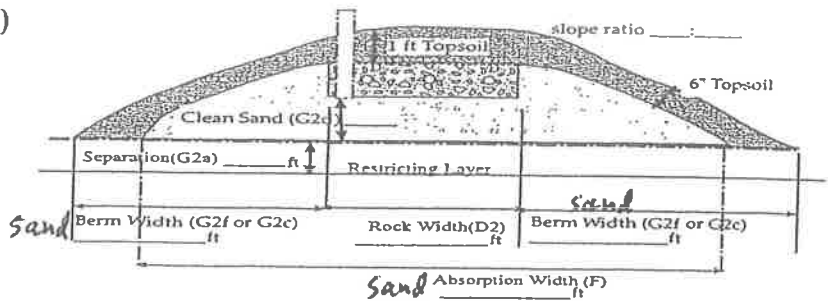


**Final Dimensions:**  
\_\_\_\_\_ X \_\_\_\_\_

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\_\_\_\_\_(signature) \_\_\_\_\_(license #) \_\_\_\_\_(date)

<=1% land slope

**G. Mound Slope Width and Length**  
(landslope less than or equal to 1%)



1. Absorption width (F) \_\_\_\_\_ ft

2. Calculate mound size

a. Determine depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)

3 ft - \_\_\_\_\_ ft = \_\_\_\_\_ ft

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

\_\_\_\_\_ ft + 1ft + 1ft = \_\_\_\_\_ ft

c. Berm width = upslope mound height (G2b) times 4 (4 is recommended, but could be 3-12)

\_\_\_\_\_ x 4 = \_\_\_\_\_ ft

d. The total landscape width is the sum of berm (G2c) width plus rock layer width (D2) plus berm width (G2c): \_\_\_\_\_ ft + \_\_\_\_\_ ft + \_\_\_\_\_ ft = \_\_\_\_\_ ft

e. Additional width necessary for absorption = absorption width (F) minus the landscape width (G2d)

\_\_\_\_\_ ft - \_\_\_\_\_ ft = \_\_\_\_\_ ft, if number is negative (<0) skip to g

f. Final berm width = additional width (G2e) plus the berm width (G2c)

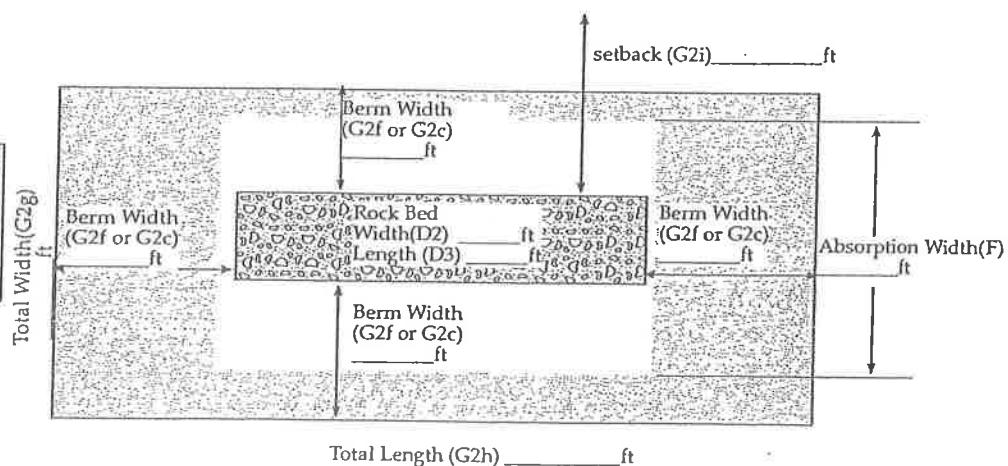
\_\_\_\_\_ ft + \_\_\_\_\_ ft = \_\_\_\_\_ ft

g. Total mound width is the sum of berm width (G2f or G2c) plus rock layer width (D2) plus berm width (G2f or G2c): \_\_\_\_\_ ft + \_\_\_\_\_ ft + \_\_\_\_\_ ft = \_\_\_\_\_ ft

h. Total mound length is the sum of berm (G2f or G2c) plus rock layer length (D3) plus berm (G2f or G2c): \_\_\_\_\_ ft + \_\_\_\_\_ ft + \_\_\_\_\_ ft = \_\_\_\_\_ ft

i. Setbacks from the rockbed are calculated as follows: the absorption width (F) minus the rock bed width (D2) divided by 2: ( \_\_\_\_\_ ft - \_\_\_\_\_ ft) ÷ 2 = \_\_\_\_\_ ft

**Final Dimensions:**  
\_\_\_\_\_ X \_\_\_\_\_



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

\_\_\_\_\_ (signature) \_\_\_\_\_ (license #) \_\_\_\_\_ (date)

BOUND CROSS-SECTION

\_\_\_\_\_ PERCENT SLOPE OF ORIGINAL SOIL \_\_\_\_\_ FT. X \_\_\_\_\_ FT. SIZE OF ROCKBED \_\_\_\_\_ FT. X \_\_\_\_\_ FT. SIZE OF SANDBASE

GEOTEXTILE CLOTH

4 INCHES OF TOPSOIL FOR GRASS COVER

14 INCHES OF SANDY LOAM SOIL TAPERING TO 8 INCHES

9" ROCK BELOW DISTRIBUTION PIPE

ORIGINAL GRADE

INCHES OF SAND \*

ROUGHENED SOIL SURFACE

INCHES OF SAND

UPSLOPE SAND WIDTH

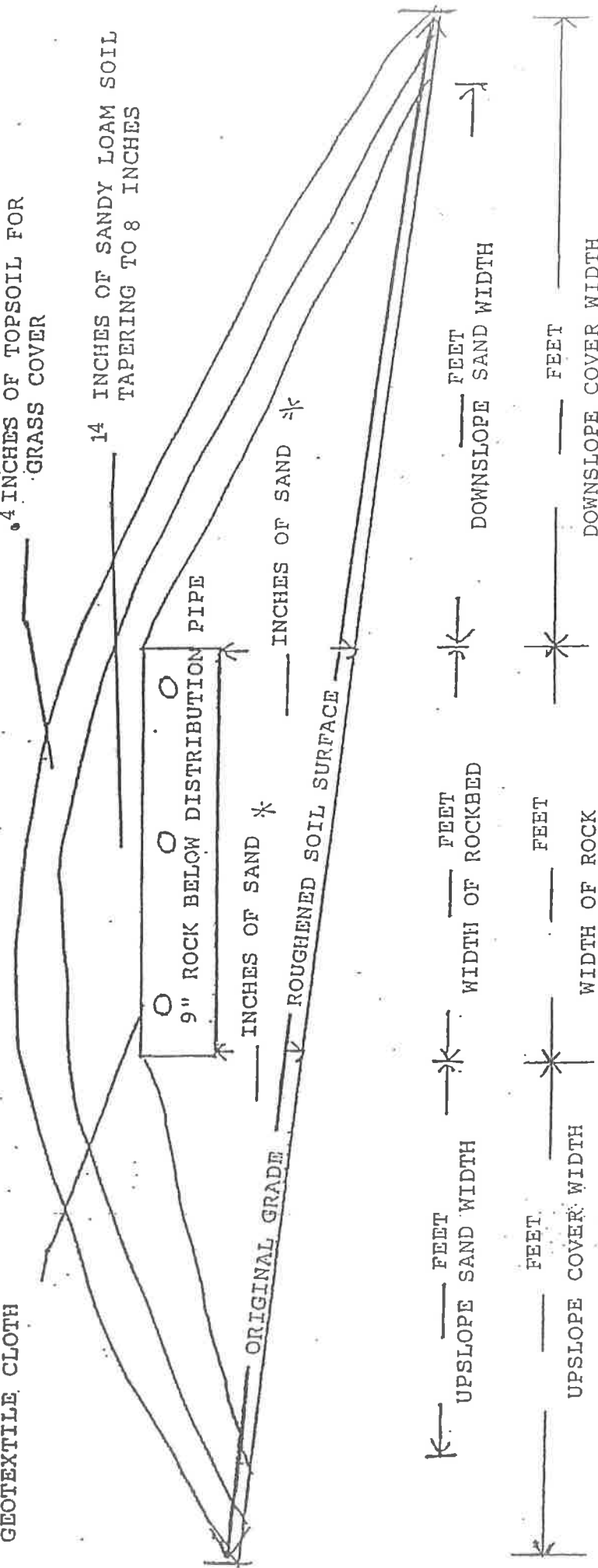
WIDTH OF ROCKBED

DOWNSLOPE SAND WIDTH

UPSLOPE COVER WIDTH

WIDTH OF ROCK

DOWNSLOPE COVER WIDTH





# PRESSURE DISTRIBUTION SYSTEM

- Select number of perforated laterals \_\_\_\_\_
- Select perforation spacing = \_\_\_\_\_ ft
- Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$$\text{Rock layer length} - 2 \text{ ft} = \text{_____ ft}$$

- Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

$$\text{Perforation spacing} = \text{_____ ft} \div \text{_____ ft} = \text{_____ spaces}$$

- Number of perforations is equal to one plus the number of perforation spaces(4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$$\text{_____ spaces} + 1 = \text{_____ perforations/lateral}$$

- A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$\text{_____ perfs/lat} \times \text{_____ lat} = \text{_____ perforations}$$

- B. Calculate the square footage per perforation. Should be 6-10 sqft/perf. Does not apply to at-grades.

Rock bed area = rock width (ft) x rock length (ft)

$$\text{_____ ft} \times \text{_____ ft} = \text{_____ sqft}$$

Square foot per perforation = Rock bed area ÷ number of perfs (6)

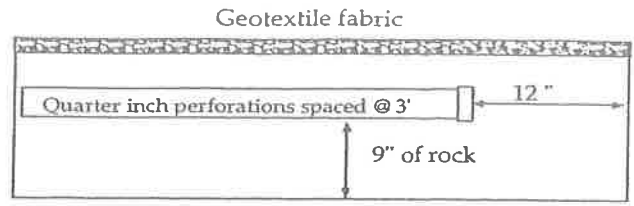
$$\text{_____ sqft} \div \text{_____ perfs} = \text{_____ sqft/perf}$$

- Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$\text{_____ perfs} \times \text{_____ gpm/perfs} = \text{_____ gpm}$$

- If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = \_\_\_\_\_ inches.

- If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = \_\_\_\_\_ inches.



Perf Sizing 3/16" - 1/4"  
Perf Spacing 1.5' - 5'

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

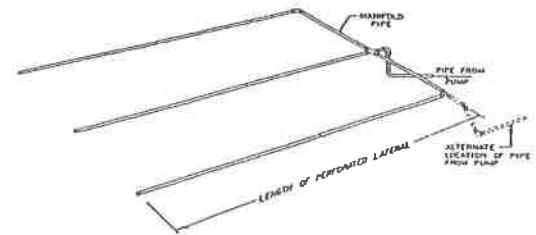
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

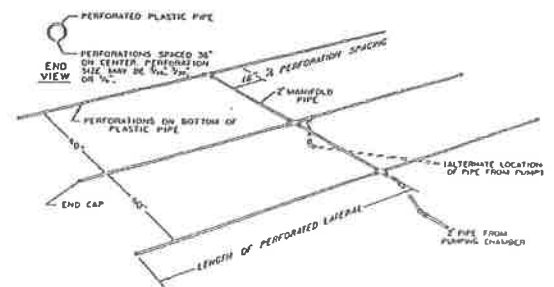
head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0 <sup>a</sup>	0.18	0.42	0.56	0.74
2.0 <sup>b</sup>	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

<sup>a</sup> Use 1.0 foot for single-family homes.  
<sup>b</sup> Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MOUND



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

\_\_\_\_\_ (signature)

\_\_\_\_\_ (license #)

\_\_\_\_\_ (date)

# PUMP SELECTION PROCEDURE

## 1. Determine pump capacity:

### A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

### B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: \_\_\_\_\_ gpm

## 2. Determine pump head requirements:

### A. Elevation difference between pump and point of discharge?

\_\_\_\_\_ feet

### B. Special head requirement? (See Figure at right - Special Head Requirements)

\_\_\_\_\_ feet

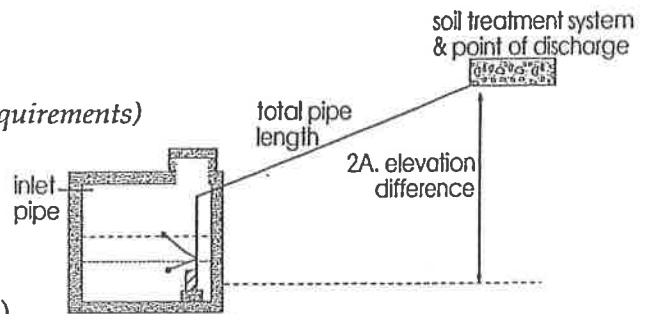
### C. Calculate Friction loss

1. Select pipe diameter \_\_\_\_\_ in
2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).  
Read friction loss in feet per 100 feet from Figure E-9  
Friction Loss = \_\_\_\_\_ ft/100ft of pipe
3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length  
\_\_\_\_\_ feet x 1.25 = \_\_\_\_\_ feet
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.  
= \_\_\_\_\_ ft/100ft x \_\_\_\_\_ ÷ 100 = \_\_\_\_\_ ft

### D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

\_\_\_\_\_ ft + \_\_\_\_\_ ft + \_\_\_\_\_ ft =

**Total head: \_\_\_\_\_ feet**



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

E-9: Friction Loss in Plastic Pipe Per 100 feet			
flow rate gpm	nominal pipe diameter		
	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

## 3. Pump selection

A pump must be selected to deliver at least \_\_\_\_\_ gpm (1A or B) with at least \_\_\_\_\_ feet of total head (2D)

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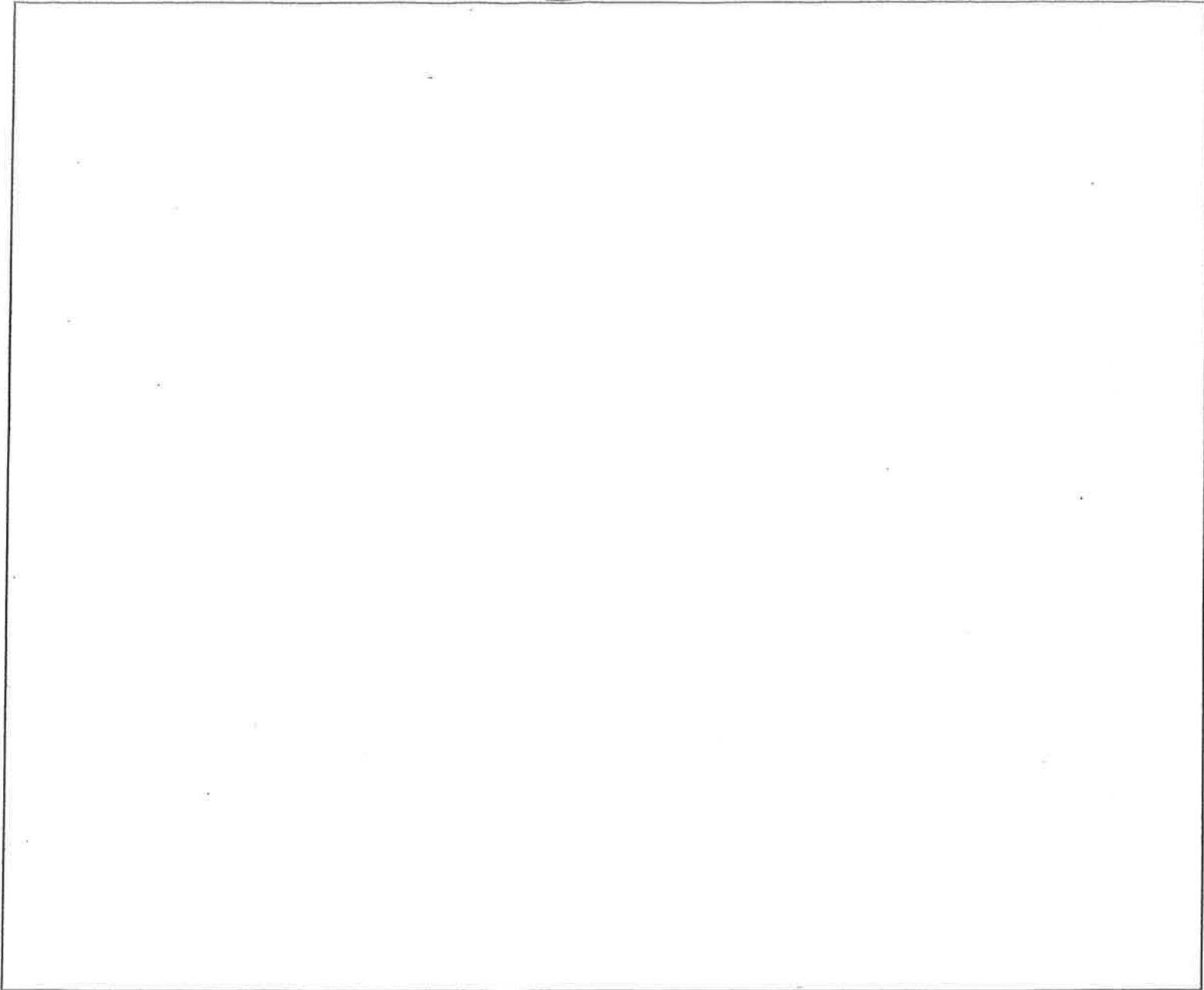
\_\_\_\_\_ (signature) \_\_\_\_\_ (license #) \_\_\_\_\_ (date)

# SKETCH SHEET

CLIENT: \_\_\_\_\_

DATE: \_\_\_\_\_

MAP DRAWN TO SCALE \_\_\_\_\_ WITH A NORTH ARROW



## CHECK OFF LIST--HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??

### SHOW EXISTING OR PROPOSED

- WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
- PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
- STRUCTURES  LOT IMPROVEMENTS
- ALL SOIL TREATMENT AREAS  ALL ISTS COMPONENTS
- HORIZONTAL AND VERTICAL REFERENCE
- POINT OF SOIL BORINGS  DIRECTION OF SLOPE
- LOT EASEMENTS  ALL LOT DIMENSIONS
- DISTURBED/ COMPACTED AREAS
- SITE PROTECTION--LATHE AND RIBBON EVERY 15 FT
- ACCESS ROUTE FOR TANK MAINTENANCE

### REQUIRED SETBACKS

- STRUCTURES  PROPERTY LINES
- OHWL

COMMENTS: \_\_\_\_\_

DESIGNER SIGNATURE \_\_\_\_\_

LICENSE# \_\_\_\_\_

### INDICATE ELEVATIONS

- \_\_\_\_\_ BENCHMARK
- \_\_\_\_\_ ELEVATION OF SEWER LINE @ HOUSE
- \_\_\_\_\_ ELEVATION @ TANK INLET
- \_\_\_\_\_ ELEVATION @ BOTTOM OF ROCK LAYER
- \_\_\_\_\_ ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER
- \_\_\_\_\_ ELEVATION OF PUMP
- \_\_\_\_\_ ELEVATION OF DISTRIBUTION DEVICE

DATE \_\_\_\_\_

# DOSING CHAMBER SIZING

1. Determine area

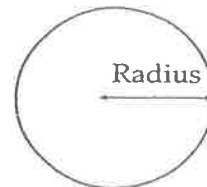
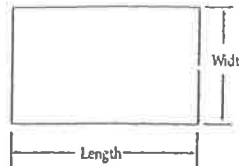
A. Rectangle area =  $L \times W$

\_\_\_\_\_ x \_\_\_\_\_ = \_\_\_\_\_ square feet

B. Circle area =  $\pi (3.14) \times \text{radius in feet} \times \text{radius in feet}$

$3.14 \times$  \_\_\_\_\_ ft  $\times$  \_\_\_\_\_ ft = \_\_\_\_\_ sqft

C. Get area from manufacturer \_\_\_\_\_ sqft



2. Calculate gallons per inch

There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.

Area  $\times 7.5 \div 12 =$  \_\_\_\_\_ sqft  $\times 7.5 \div 12 \text{ in/ft} =$  \_\_\_\_\_ gallon per inch

3. Calculate total tank volume

A. Depth from bottom of inlet pipe to tank bottom \_\_\_\_\_ in

B. Total tank volume = depth from bottom of inlet pipe to tank bottom (3A)  $\times$  gal/in (2)  
= \_\_\_\_\_ in  $\times$  \_\_\_\_\_ gal/in = \_\_\_\_\_ gal

4. Calculate gallons to cover pump (with 2-3 inches of water covering pump)

(Pump and block height (inch) + 2 inch)  $\times$  gallon/inch

(\_\_\_\_\_ in + 2 in)  $\times$  \_\_\_\_\_ gal/in = \_\_\_\_\_ gallon

5. Calculate total pumpout volume

A. Select pump size for 4-5 does per day. Gallon per dose = gpd (see figure A-1) / doses per day = \_\_\_\_\_ gpd  $\div$  \_\_\_\_\_ doses/day = \_\_\_\_\_ gallons

B. Calculate drainback

1. Determine total pipe length, \_\_\_\_\_ feet

2. Determine liquid volume of pipe, \_\_\_\_\_ gal per ft (see figure E-20)

3. Drainback quantity = \_\_\_\_\_ ft (5B1)  $\times$  \_\_\_\_\_ gal per ft (5B2) = \_\_\_\_\_ gal

C. Total pump out volume = dose volume (5A) + drainback (5B3)

\_\_\_\_\_ gal + \_\_\_\_\_ gal = \_\_\_\_\_ Total gallon

6. Float separation distance (using total pumpout volume)

Total pumpout volume (5C)  $\div$  gal/inch (2)

\_\_\_\_\_ gal  $\div$  \_\_\_\_\_ gal/in = \_\_\_\_\_ inch

7. Calculate volume for alarm (typically 2 to 3 inches)

Alarm depth (inch)  $\times$  gallon/inch (2) = \_\_\_\_\_ in  $\times$  \_\_\_\_\_ gal/in = \_\_\_\_\_ gal

8. Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)

\_\_\_\_\_ gal + \_\_\_\_\_ gal + \_\_\_\_\_ gal = \_\_\_\_\_ gallons

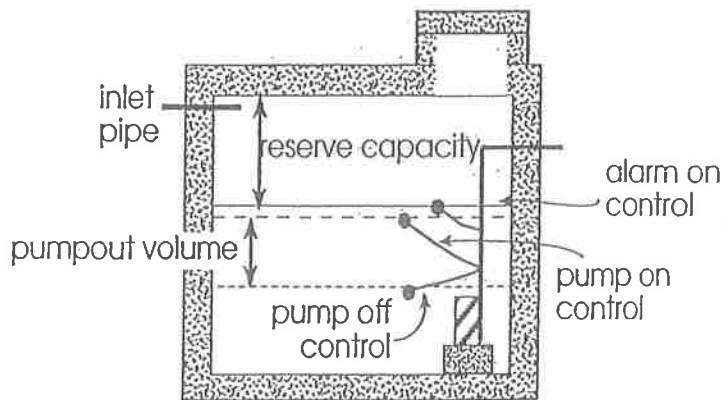
9. Total Tank Depth = total gallon (8)  $\div$  gallon/inch (2)

\_\_\_\_\_ gal  $\div$  \_\_\_\_\_ gal/in = \_\_\_\_\_ in

**Legal Tank:**  
**500 gallons or**  
**100% the Daily flow**  
**or**  
**Alternating Pumps**

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

Pipe Diameter inches	Gallons per foot
1	0.045
1.25	0.078
1.5	0.11
2	0.17
2.5	0.25
3	0.38
4	0.66



**Recommended:**

Calculate reserve capacity (75% the daily flow)

Daily flow  $\times .75 =$  \_\_\_\_\_  $\times .75 =$  \_\_\_\_\_ gallons

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

\_\_\_\_\_ (signature)

\_\_\_\_\_ (license #)

\_\_\_\_\_ (date)

# ATGRADE DESIGN WORK SHEET

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I
7	1050	600	370	2, or 3
8	1200	675	408	columns

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

### A. AVERAGE DESIGN FLOW

Estimated \_\_\_\_\_ gpd (see figure A-1)  
 or measured \_\_\_\_\_ x 1.5 (safety factor) = \_\_\_\_\_ gpd

### B. SEPTIC TANK CAPACITY

\_\_\_\_\_ gallons (see figure C-1)

### C. SOILS (refer to site evaluation)

1. Depth to restricting layer = \_\_\_\_\_ feet
2. Depth of percolation tests = \_\_\_\_\_ feet
3. Texture \_\_\_\_\_ Percolation rate \_\_\_\_\_ mpi
4. SSF \_\_\_\_\_ ft<sup>2</sup>/gpd (see figure D-15)
5. Linear Loading Rate \_\_\_\_\_ gpd/ft (see figure D-42)
6. Land slope \_\_\_\_\_ %

### D. ROCK WIDTH

1. Rock absorption width equals LLR (C5) times SSF (C4) = \_\_\_\_\_ gpd/ft x \_\_\_\_\_ ft<sup>2</sup>/gpd = \_\_\_\_\_ ft

### E. SYSTEM SIZE

1. The height of the system is 2 feet
2. Determine upslope width
  - a. Upslope multiplier based on % land slope (see figure D-46): \_\_\_\_\_
  - b. Upslope width = upslope multiplier (E2a) times height (E1)  
 \_\_\_\_\_ x 2 ft = \_\_\_\_\_ ft
3. Determine downslope width
  - a. Downslope multiplier based on % land slope (see figure D-46) \_\_\_\_\_
  - b. Downslope width = downslope multiplier (E2a) by height (E1)  
 \_\_\_\_\_ x 2 ft = \_\_\_\_\_ ft
  - c. Rock absorption width (D1) + 5' = \_\_\_\_\_ + 5' = \_\_\_\_\_ ft
  - d. Downslope width equals larger of 3a and 3b \_\_\_\_\_ ft
4. System width is the sum of upslope width (E2b) plus downslope width (E3d):  
 \_\_\_\_\_ ft + \_\_\_\_\_ ft = \_\_\_\_\_ feet
5. The rock layer length is the flow (A) divided by LLR (C5)  
 \_\_\_\_\_ gpd ÷ \_\_\_\_\_ gpd/ft = \_\_\_\_\_ feet
6. Total length is the sum of upslope width (E2b) plus rock layer length (E5) plus upslope width (E2b)  
 \_\_\_\_\_ ft + \_\_\_\_\_ ft + \_\_\_\_\_ ft = \_\_\_\_\_ feet

### F. ROCK VOLUME

1. Rock Area = Length(E5) x Width(D1+1ft)  
 \_\_\_\_\_ ft x (\_\_\_\_\_ ft + 1ft) = \_\_\_\_\_ sqft
2. Multiply rock area(F1) by depth of rock(1 ft) and divide by two because the shape is triangular  
 \_\_\_\_\_ sqft x 1 ft ÷ 2 = \_\_\_\_\_ cuft
3. Divide cubic feet by 27 cuft/cuyd to get cubic yards  
 \_\_\_\_\_ cuft ÷ 27 = \_\_\_\_\_ cuyd
4. Multiply cubic yards by 1.4 to get weight of rock in tons  
 \_\_\_\_\_ cuyd x 1.4 = \_\_\_\_\_ tons

Percolation Rate at 12" (MP)	Soil Texture (0-12")	Other Characteristics in Upper 48"	Linear Loading Rate - LLR (gpd/ft)
Faster than 0.1	Coarse Sand	No textural change Layers of other textures	6
		Saturated soil (<3') Bedrock	5
0.1 to 5	Sand Loamy Sand Fine Sand*	No textural change Layers of other textures	8
		Layers or Banding	4
		Saturated soil (<3') Bedrock	5
		Condition III	7
6 to 15	Sandy Loam	Strong to moderate structure No textural change	6
		Condition II	5
		Condition III	4
16 to 60	Lean Silt Loam Silt Sandy Clay Loam Silty Clay Loam Clay Loam	Strong to moderate structure No textural change	6
		Condition II	5
		Condition III	4
		Condition III	3
60 to 120 Slower than 120	Clay Sandy Clay Silty Clay	Strong or moderate structure No textural change	3
		Condition II	2
		Condition III	2

Condition II weak structure layers of other textures      Condition III platy or massive structure saturated soil (<3') or bedrock (<4')

Total system LLR < 8

Percolation Rate (minutes per inch (mpi))	Soil Texture	Soil Sizing Factor (square feet/gallon per day (sqft/gpd))
faster than 0.1*	Coarse sand	0.83
0.1 to 5	Medium sand	0.83
0.1 to 5**	Loamy sand	1.67
	Fine sand	1.27
6 to 15	Sandy loam	1.67
16 to 30	Loam	2.00
31 to 45	Silt loam	2.00
46 to 60	Silt	2.20
	Clay loam	
	Sandy clay	
over 61 to 120***	Silty clay	4.20
	Clay	
	Sandy clay Silty clay	
slower than 120****		

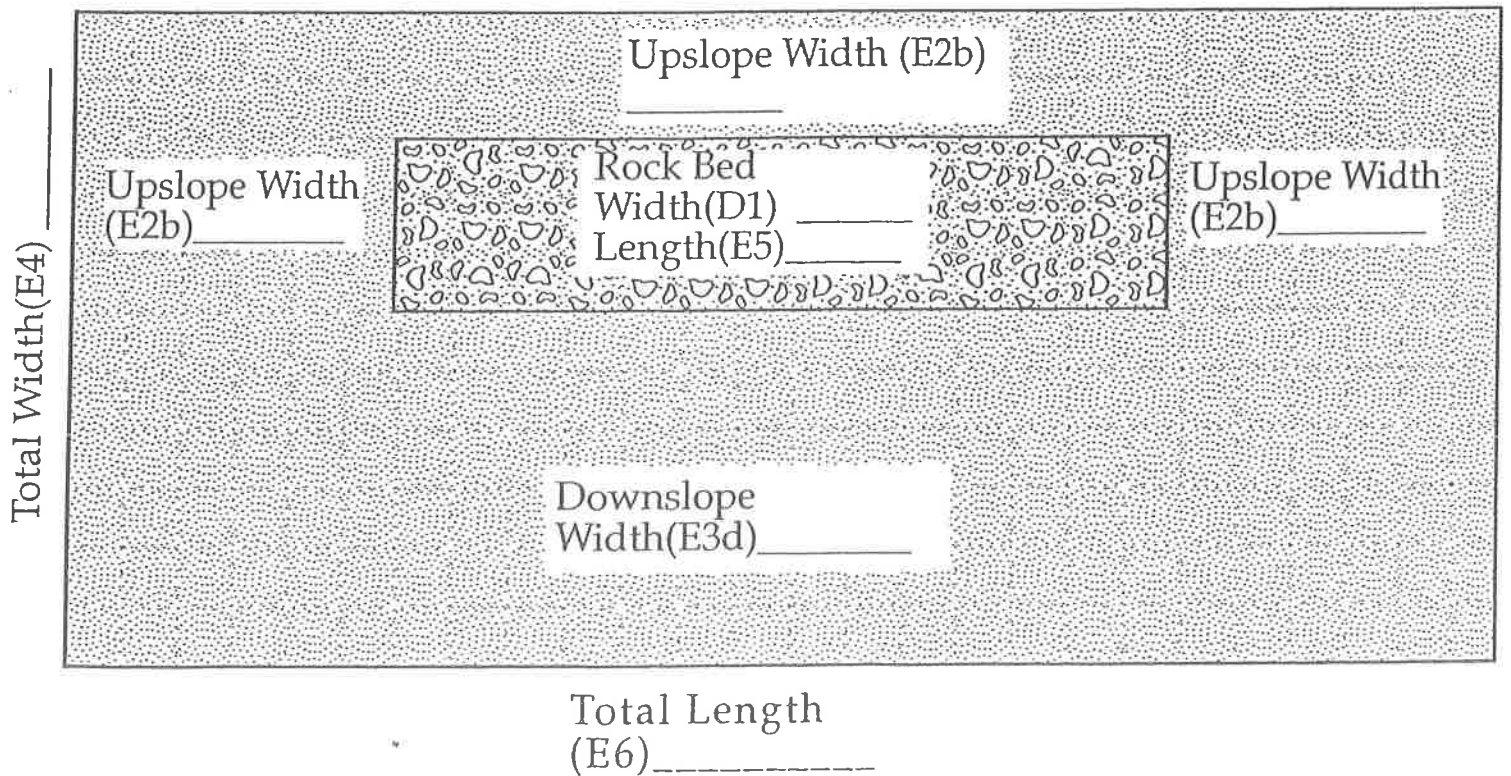
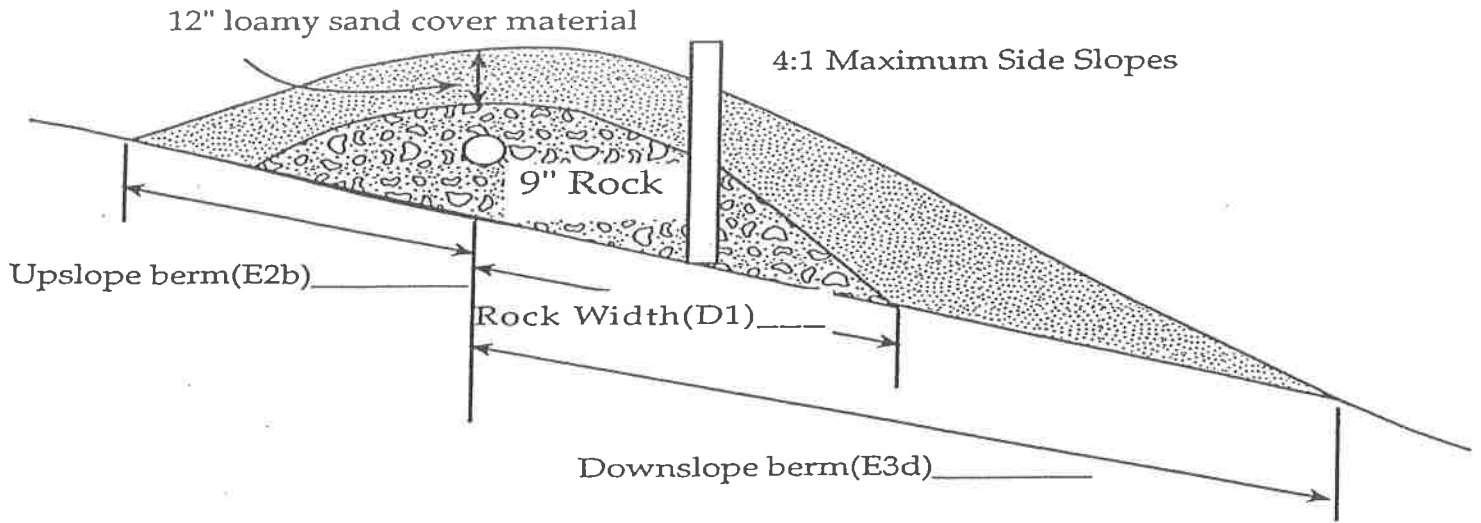
\*Use systems for rapidly permeable soils: pressure distribution or serial distribution with no trench >25% of the total system.  
 \*\*Soil having 50% or more fine sand plus very fine sand  
 \*\*\*A mound must be used.  
 \*\*\*\*An other or performance system must be used

Land Slope, in %	UPSLOPE berm multipliers for berm slope ratio of 4:1	DOWNSLOPE berm multipliers for berm slope ratio of 4:1
0	4.0	4.0
1	3.85	4.17
2	3.70	4.35
3	3.57	4.54
4	3.45	4.76
5	3.33	5.00
6	3.23	5.26
7	3.12	5.56
8	3.03	5.88
9	2.94	6.25
10	2.86	6.67
11	2.78	7.14
12	2.70	7.69

\* 4:1 is the steepest slope allowed, less steeper slopes may be used, see Fig D-34 in manual

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\_\_\_\_\_ (signature)      \_\_\_\_\_ (license #)      \_\_\_\_\_ (date)

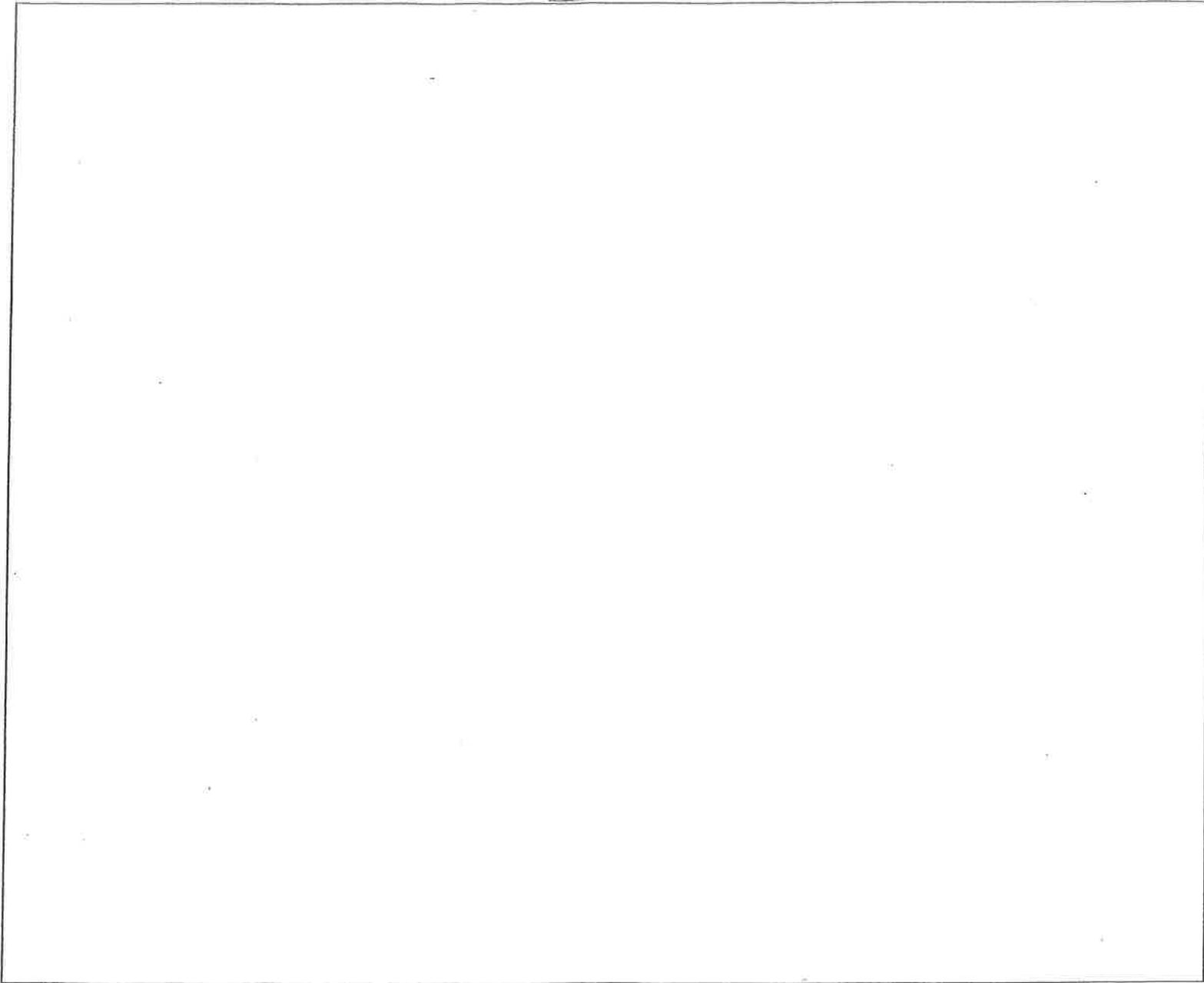


SKETCH SHEET

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MAP DRAWN TO SCALE \_\_\_\_\_ WITH A NORTH ARROW



**CHECK OFF LIST--HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??**

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- WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
- PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
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- ALL SOIL TREATMENT AREAS  ALL ISTS COMPONENTS
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- SITE PROTECTION--LATHE AND RIBBON EVERY 15 FT
- ACCESS ROUTE FOR TANK MAINTENANCE

**REQUIRED SETBACKS**

- STRUCTURES  PROPERTY LINES
- OHWL

COMMENTS:

DESIGNER SIGNATURE \_\_\_\_\_

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**INDICATE ELEVATIONS**

- \_\_\_\_\_ BENCHMARK
- \_\_\_\_\_ ELEVATION OF SEWER LINE @ HOUSE
- \_\_\_\_\_ ELEVATION @ TANK INLET
- \_\_\_\_\_ ELEVATION @ BOTTOM OF ROCK LAYER
- \_\_\_\_\_ ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER
- \_\_\_\_\_ ELEVATION OF PUMP
- \_\_\_\_\_ ELEVATION OF DISTRIBUTION DEVICE

DATE \_\_\_\_\_